

APPENDIX II

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS
 HALTER MARINE SERVICES, INC.
 CRANE PAVING AND MISCELLANEOUS IMPROVEMENTS
 LOCKPORT, LOUISIANA

(Sheet 1 of 2)



Ground Elev.:

Datum:

Gr. Water Depth: See Text

Job No.: 15405

Date Drilled: 5/20/98

Boring: 5

Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests			
										Dry	Wet	Type	ϕ	C	LL	PL	PI					
0	45	17		X	Compact gray clayey silt w/shell fragments	ML	1	0-0.5	9													
					Very stiff tan & brown silty clay	CL	2	1-2														
					Medium stiff gray silty clay	CL	3	3-4														
				T	Medium compact gray sandy silt	ML	4	5-6	25	94	118	UC	-	435								
							5	8-9	34	86	115	UC	-	515								
							6	11-12	29	94	121											
							7	14-15														
							8	18-19	30	89	116							29	24	5		
							9	23-24														
							10	28-29	33	84	112	UC	-	265								
							11	31-32														
				T	Soft gray silty clay	CL	12	34-35	29	90	116	OB	--	670								
					Loose gray clayey silt	ML	13	38-39														
					Medium dense gray clayey sand	SC	14	43-44	30	85	110	OB	-	525								
					Medium stiff gray sandy clay	CL	15	48-49	20													
					Loose gray silty sand w/clay layers	SM																

Comments:

EUSTIS ENGINEERING COMPANY, INC.

LOG OF BORING AND TEST RESULTS
 HALTER MARINE SERVICES, INC.
 CRANE PAVING AND MISCELLANEOUS IMPROVEMENTS
 LOCKPORT, LOUISIANA

(Sheet 2 of 2)



Ground Elev.:

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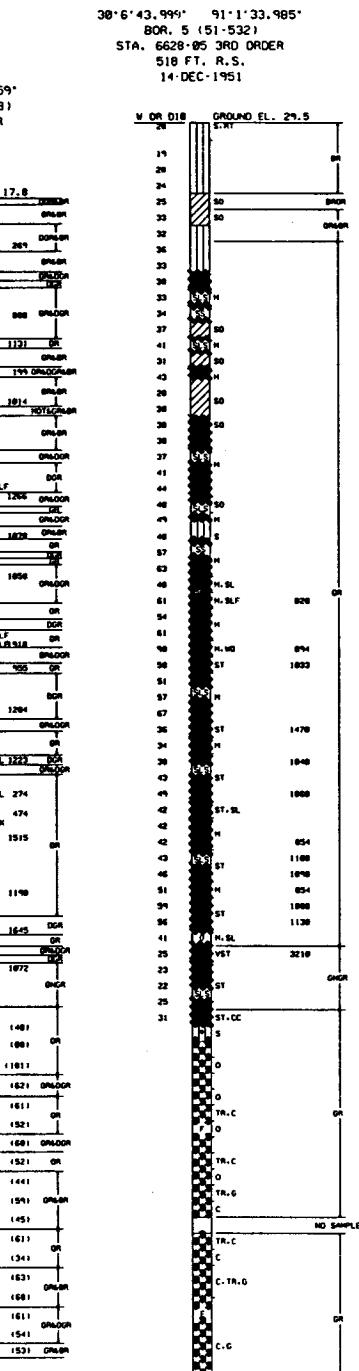
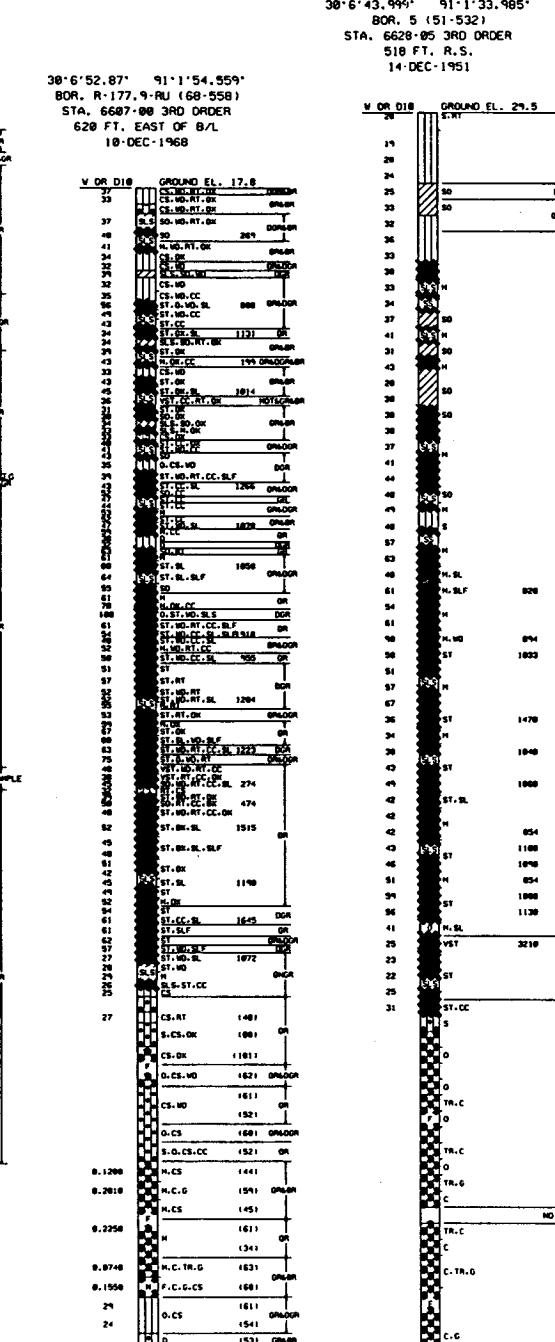
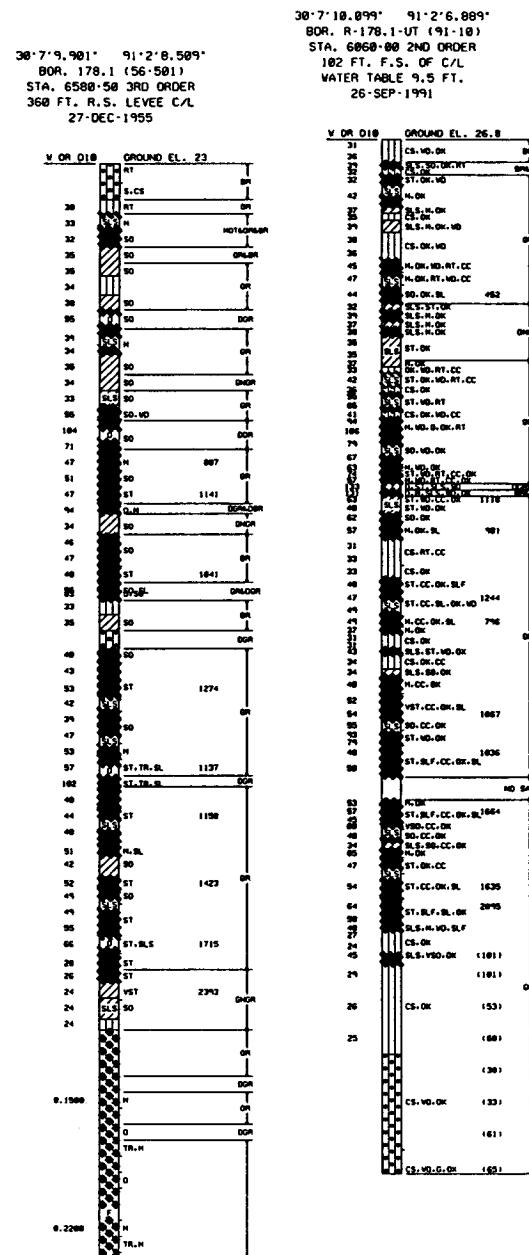
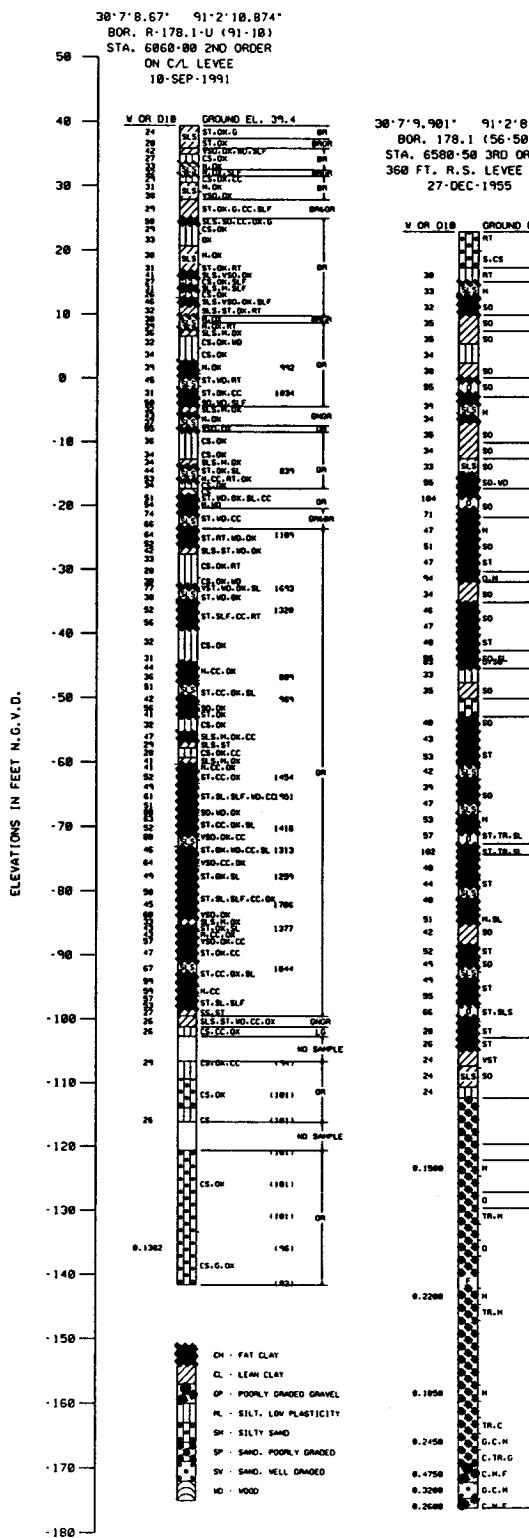
Refer to "Legends & Notes"

Scale In Feet	PP	SPT	S P L R	Symbol	Visual Classification	USC	Sample Number	Depth In Feet	Water Content Percent	Density		Shear Tests			Atterberg Limits				Other Tests			
										Dry	Wet	Type	ϕ	C	LL	PL	PI					
50	0.25	8	T	X	Loose gray silty sand w/clay layers	SM																
					Medium stiff gray clay w/shell fragments & sand lenses	CH				16	53-54	51	69	105	UC	-	790					
										17	58-59											
					Medium stiff gray silty clay	CL				18	63-64	34	82	110	UC	-	535					
					Medium stiff gray clay	CH				19	68-69											
										20	73-74	54	67	103	UC	--	720					

Area 1

(1 of 1)

1' = 30'



30°6'39.388" 91°1'31.182"
BOR. R-177.5-U (91-18)
STA. 6110-00 2ND ORDER
C/L LEVEE
WATER TABLE 14.8 FT.
28-AUG-1991

17

CS-RT

SL-RT

VS-RT

SH-RT

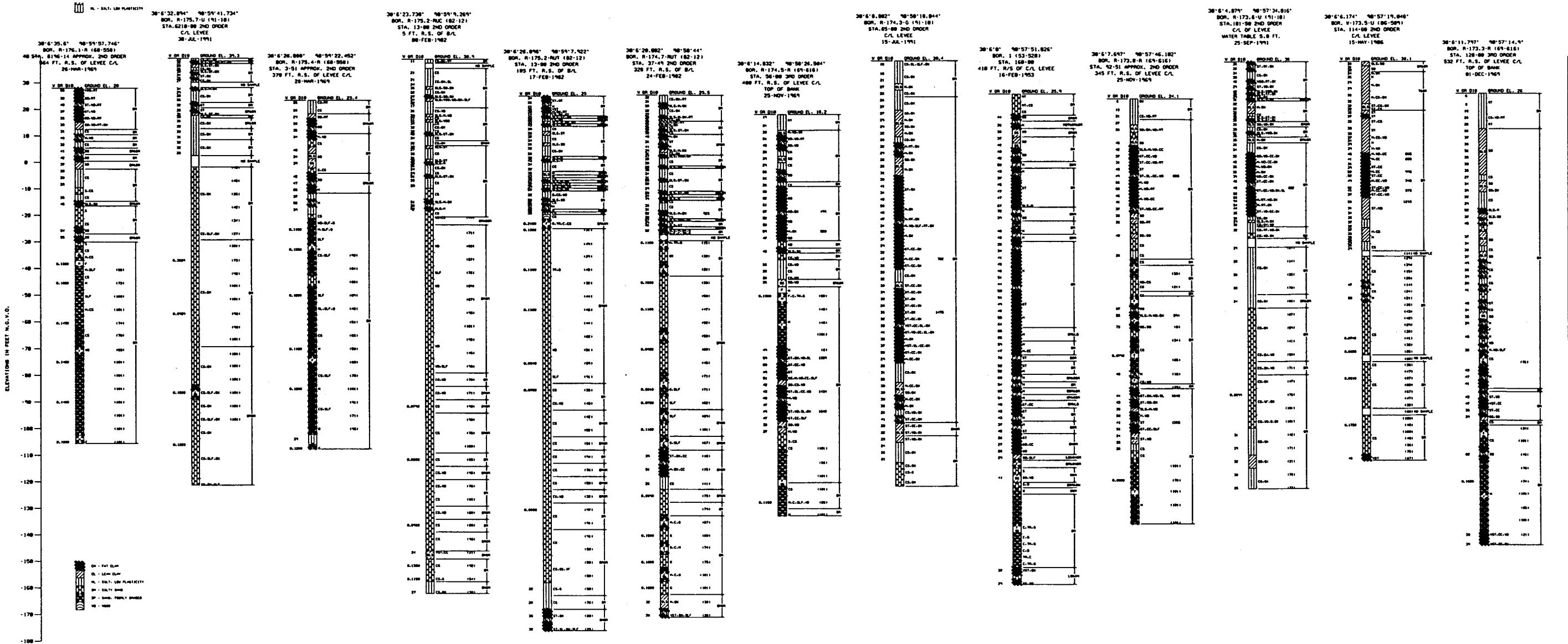
CC-RT

SL-RT

HO-RT

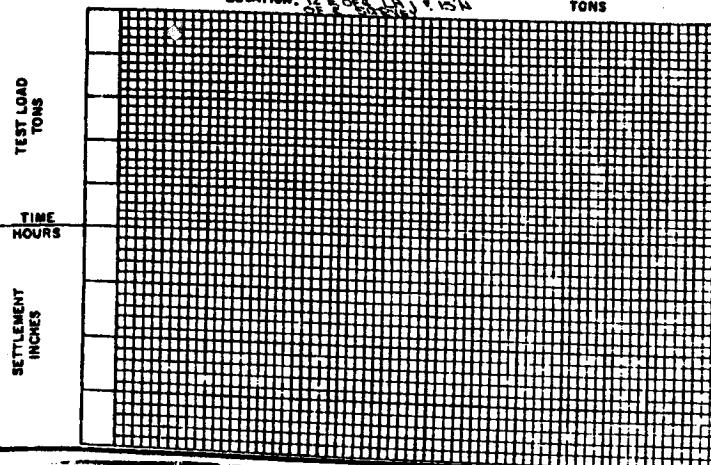
Area 3 (1 of 1)

$$l'' = 40'$$

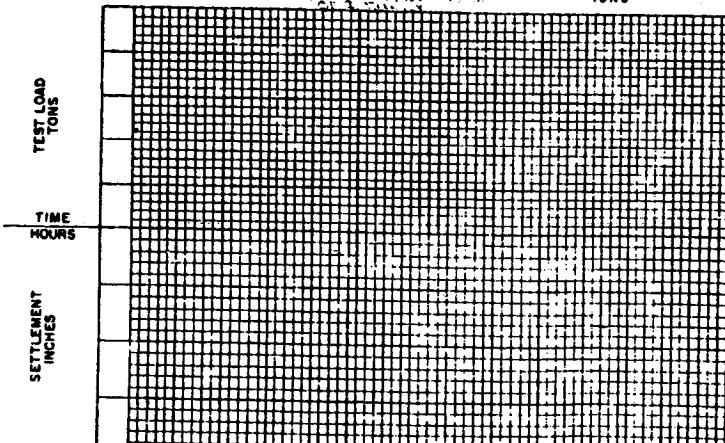


SOIL TYPE & COLOR	TEST PILE No.								
	4"	c	TYPE	BREAK	MOISTURE	LIQUID	PLASTICITY	INDEX	SAMPLE
BR. S. CL. WTR. RD.	-	..	C.S.	21	45	22	(5)	15	STA
BR. S. CL. WTR. RD.	40	45	OSI	52	39	11	(2)	10	
BR. S. CL. WTR. RD.	50	60	OSI	29	28	2	(3)	5	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	0	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-5	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-10	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-15	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-20	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-25	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-30	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-35	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-40	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-45	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-50	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-55	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-60	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-65	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-70	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-75	
WTR. SH. & OCS. @ 75' NOC	"	"	"	"	"	"	"	"	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-80	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-85	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-90	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-95	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-100	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-105	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-110	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-115	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-120	
WTR. SH. & OCS. @ 75' NOC	100	60	OSI	29	28	2	(3)	-125	

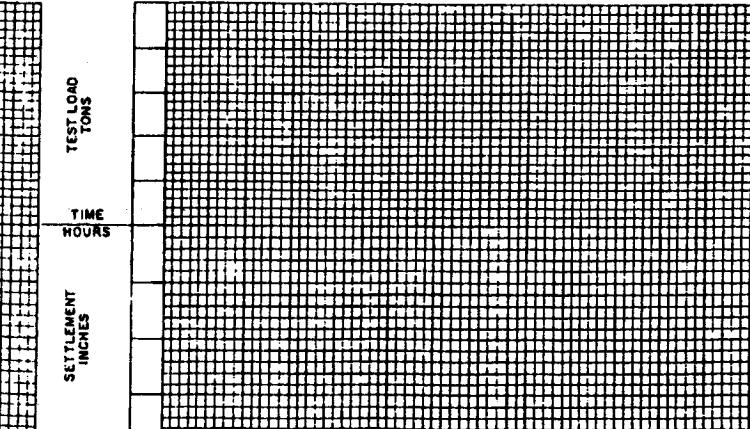
BORING NR. 1 DATE MADE 3-2-61 STA: 103+80.15 LOCATION: 1/2 E OF STA 103+80.15 DRIVING RESISTANCE TONS



BORING NR. 2 DATE MADE: 3-2-61 STA: 103+80.15 LOCATION: 1/2 E OF STA 103+80.15 DRIVING RESISTANCE TONS



BORING NR. DATE MADE: STA: LOCATION: DRIVING RESISTANCE TONS

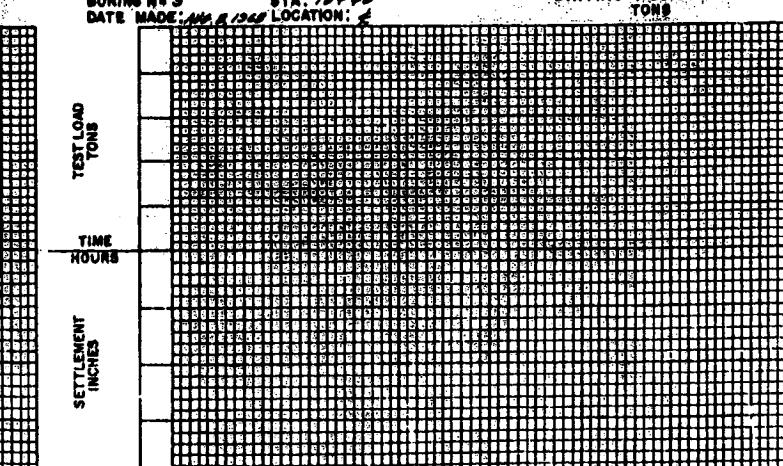
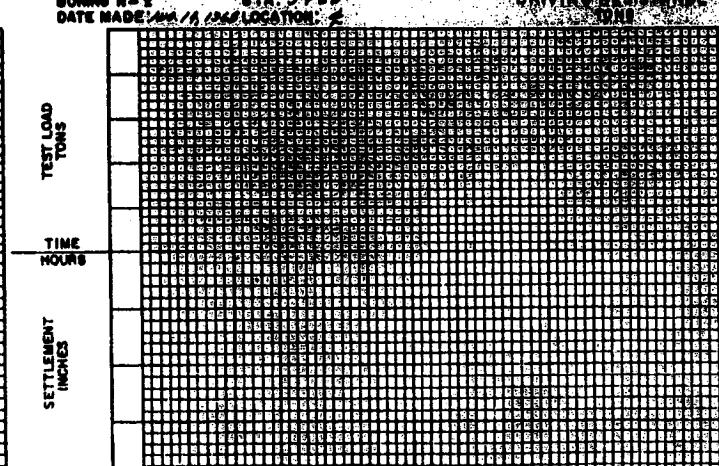
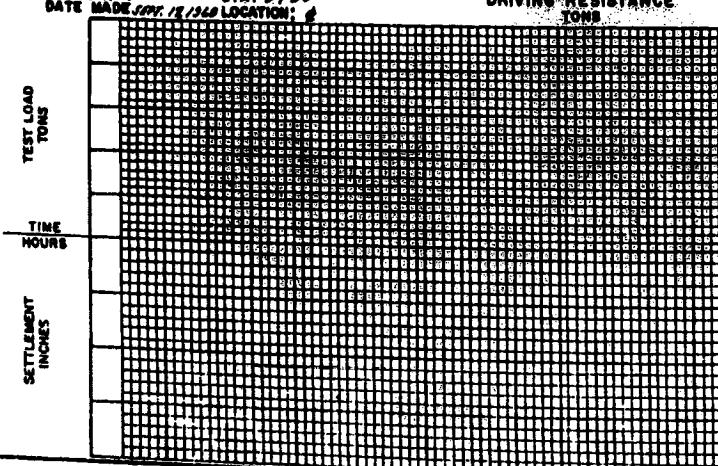


AS BUILT

SOIL TYPE & COLOR	TEST PILE No.								
	4"	c	TYPE	BREAK	MOISTURE	LIQUID	PLASTICITY	INDEX	SAMPLE
GR. S. CL. WTR. RD.	-	..	C.S.	21	45	22	(5)	15	STA
GR. S. CL. WTR. RD.	40	45	OSI	52	39	11	(2)	10	
GR. S. CL. WTR. RD.	50	60	OSI	29	28	2	(3)	5	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	0	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-5	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-10	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-15	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-20	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-25	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-30	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-35	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-40	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-45	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-50	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-55	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-60	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-65	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-70	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-75	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-80	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-85	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-90	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-95	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-100	

SOIL TYPE & COLOR	TEST PILE No.								
	4"	c	TYPE	BREAK	MOISTURE	LIQUID	PLASTICITY	INDEX	SAMPLE
GR. S. CL. WTR. RD.	-	..	C.S.	21	45	22	(5)	15	STA
GR. S. CL. WTR. RD.	40	45	OSI	52	39	11	(2)	10	
GR. S. CL. WTR. RD.	50	60	OSI	29	28	2	(3)	5	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	0	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-5	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-10	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-15	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-20	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-25	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-30	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-35	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-40	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-45	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-50	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-55	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-60	
GR. S. CL. WTR. RD.	100	60	OSI	29	28	2	(3)	-65	
GR. S. CL. WTR. RD.	100								

CLASSIFICATION				TEST FILE NO.			
SOIL TYPE & COLOR	GR.	CONSISTENCY	MOISTURE CONTENT	Liquid Limit	Plasticity Index	Sample No.	
SILTY CLAY LOAM			9	27	6	10	20
SILTY CLAY LOAM			10	44	4	11	21
SILTY CLAY LOAM	STAB	13	31	11			
SL. SILTY TO SANDY CLAY LOAM	STAB	16	23	6			
SL. SILTY CLAY - DR. LOAM	MUD	29	33	15			
SL. SILTY TO SANDY CLAY LOAM	0.75	MUD	30	36	9		
SL. SILTY WITH CLAY LOAM	0.65	MUD	29	37	6		
SL. SILT WITH LOAM		LOAM	30	41	11	12	22
SL. SILT WITH CLAY LOAM		MUD	29	41	11	13	23
SL. SILT WITH CLAY LOAM		MUD	28	41	11	14	24
SL. SILTY CLAY LOAM		MUD	29	41	11	15	25
SL. SILTY CLAY LOAM		MUD	30	41	11	16	26
SL. SILTY CLAY LOAM		MUD	29	41	11	17	27
SL. SILTY CLAY LOAM		MUD	29	41	11	18	28
SL. SILTY CLAY LOAM		MUD	29	41	11	19	29
SL. SILTY CLAY LOAM		MUD	29	41	11	20	30
SL. SILTY CLAY LOAM		MUD	29	41	11	21	31
SL. SILTY CLAY LOAM		MUD	29	41	11	22	32
SL. SILTY CLAY LOAM		MUD	29	41	11	23	33
SL. SILTY CLAY LOAM		MUD	29	41	11	24	34
SL. SILTY CLAY LOAM		MUD	29	41	11	25	35
SL. SILTY CLAY LOAM		MUD	29	41	11	26	36
SL. SILTY CLAY LOAM		MUD	29	41	11	27	37
SL. SILTY CLAY LOAM		MUD	29	41	11	28	38
SL. SILTY CLAY LOAM		MUD	29	41	11	29	39
SL. SILTY CLAY LOAM		MUD	29	41	11	30	40
SL. SILTY CLAY LOAM		MUD	29	41	11	31	41
SL. SILTY CLAY LOAM		MUD	29	41	11	32	42
SL. SILTY CLAY LOAM		MUD	29	41	11	33	43
SL. SILTY CLAY LOAM		MUD	29	41	11	34	44
SL. SILTY CLAY LOAM		MUD	29	41	11	35	45
SL. SILTY CLAY LOAM		MUD	29	41	11	36	46
SL. SILTY CLAY LOAM		MUD	29	41	11	37	47
SL. SILTY CLAY LOAM		MUD	29	41	11	38	48
SL. SILTY CLAY LOAM		MUD	29	41	11	39	49
SL. SILTY CLAY LOAM		MUD	29	41	11	40	50
SAND			28	41	11	41	
SAND			28	41	11	42	
SAND			28	41	11	43	
SAND			28	41	11	44	
SAND			28	41	11	45	
SAND			28	41	11	46	
SAND			28	41	11	47	
SAND			28	41	11	48	
SAND			28	41	11	49	
SAND			28	41	11	50	



NOTES:	R.A.R.	STATE PROJECT	P.
150	D-500 (D)	ESB-30-01	ASS

ABBREVIATIONS:

N.P.	NON-PLASTIC
BL.	BLUE
BK.	BLACK
BR.	BROWN
GR.	GRAY
WH.	WHITE
YE.	YELLOW
DK.	DARK
LT.	LIGHT
MED.	MEDIUM
PENCIL	PENETRATION
S/B	UNABLE TO TEST BECAUSE OF SLICKENRIDES

MOISTURE CONTENT = MOISTURE CONTENT OF THE MATERIAL
IN ITS NATURAL STATE EXPRESSED
AS A PERCENTAGE OF THE DRY WEIGHT
OF THE MATERIAL.

NO PENETRATION : UNABLE TO DRIVE SPLIT SPOON SAMPLER INITIAL 6 INCHES.

N-10

NUMBER OF BLOWS OF 100 LB.
HAMMER DROPPED 30 IN. REQUIRED
TO DRIVE 2 IN. SPLIT SPOON SAMPLER
1 FT. AFTER FIRST HAVING BEEN DRIVEN
6 IN. UNLESS AMOUNT OF PENETRATION
SHOWN OTHERWISE.

NO CULL • NO PRELIMINARY 6" DRIVING PRIOR TO SECURING DRIVE DATA.

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES				
SOIL	DESIGNATION	NO OF BLOWS "N"	"N"	INCONFINED COMPRESSIVE STRENGTH TONS PER SQ.
SAND OR SILT	VERY LOOSE		LESS THAN 4	
	LOOSE		4-10	
	MEDIUM		10-30	
	DENSE		30-50	
CLAY	VERY DENSE		OVER 50	
	SOFT		LESS THAN 2	LESS THAN
	SOFT		2-4	0.25-0.50
	MEDIUM		4-6	0.50-1.00
	STIFF		6-15	> 1.00
	STIFF		15-30	> 2
	HARD		OVER 30	OVER 4

CORE BORINGS AND TEST PILES. BRIDGE NO. I

BAYOU LAFOURCHE BRIDGE
INCOURTVILLE - SUNSHINE BRIDGE HWY
La 70

205 " 168

STATE OF LOUISIANA
DEPARTMENT OF HIGHWAYS

TESTING SECTION - BRIDGE DESIGN SECTION

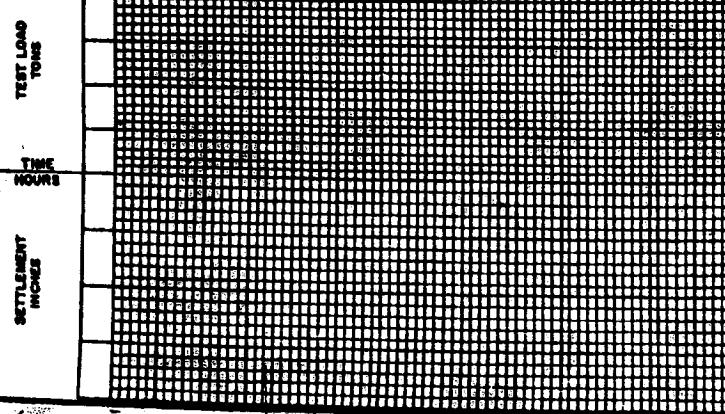
SHEET 1 OF 7

PLATE I

SOIL TYPE & COLOR	QU	QT	TEST PILE No.		ELEVATION	STA.
			TYPE OF PILE:	TYPE OF HAMMER:		
GR. CLY. SILT	0.15	HR.	AST 27	SL 12	15	15
W/JTR. 020.5%						
GR. CLY. SOY. SILT	0.10	HR.	AST 23	N. P.	10	
GR. SIC. CL.	0.05	HR.	AST 31	SL 7	5	
GR. CLY. SOY. SILT	1.15	HR.	AST 31	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.80	HR.	AST 32	SL 11	10	
GR. CLY. SI. SA.	1.10	HR.	AST 23	N. P.	15	
GR. CLY. SOY. SILT	0.05	HR.	AST 30	SL 3	5	
GR. SIC. CL.	0.70	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. CLY. SOY. SILT	0.05	HR.	AST 30	SL 3	5	
GR. SIC. CL.	0.70	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.60	HR.	AST 30	SL 6	10	
GR. CL.	0.50	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.60	HR.	AST 30	SL 6	10	
GR. CL.	0.50	HR.	AST 30	SL 6	10	
GR. CLY. SILT	0.80	HR.	AST 31	SL 16	15	

BORING NO. STA: 141.86
DATE MADE: 3-8-71 LOCATION: 102-76

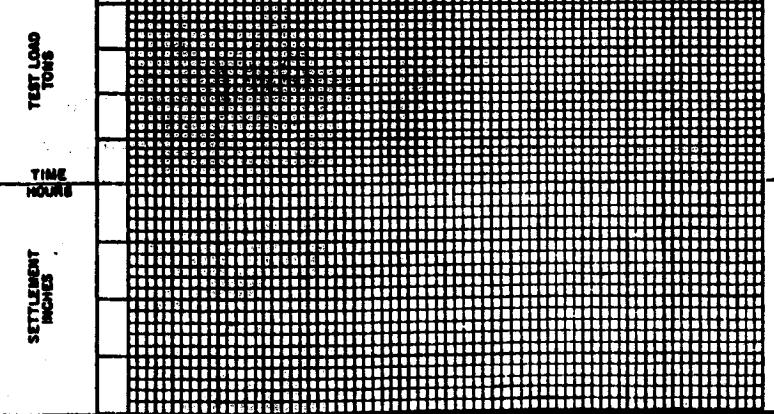
DRIVING RESISTANCE
TONS



SOIL TYPE & COLOR	QU	QT	TEST PILE No.		ELEVATION	STA.
			TYPE OF PILE:	TYPE OF HAMMER:		
GR. CLY. SILT	0.15	HR.	AST 27	SL 12	15	15
W/JTR. 020.5%						
GR. CLY. SOY. SILT	0.10	HR.	AST 23	N. P.	10	
GR. SIC. CL.	0.05	HR.	AST 31	SL 7	5	
GR. CLY. SOY. SILT	1.15	HR.	AST 31	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.80	HR.	AST 32	SL 11	10	
GR. CLY. SI. SA.	1.10	HR.	AST 23	N. P.	15	
GR. CLY. SOY. SILT	0.05	HR.	AST 30	SL 3	5	
GR. SIC. CL.	0.70	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. CLY. SOY. SILT	0.05	HR.	AST 30	SL 3	5	
GR. SIC. CL.	0.70	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.60	HR.	AST 30	SL 6	10	
GR. CL.	0.50	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.60	HR.	AST 30	SL 6	10	
GR. CL.	0.50	HR.	AST 30	SL 6	10	
GR. CLY. SILT	0.80	HR.	AST 27	SL 16	15	

BORING NO. STA: 141.76
DATE MADE: 3-8-71 LOCATION: 102-76

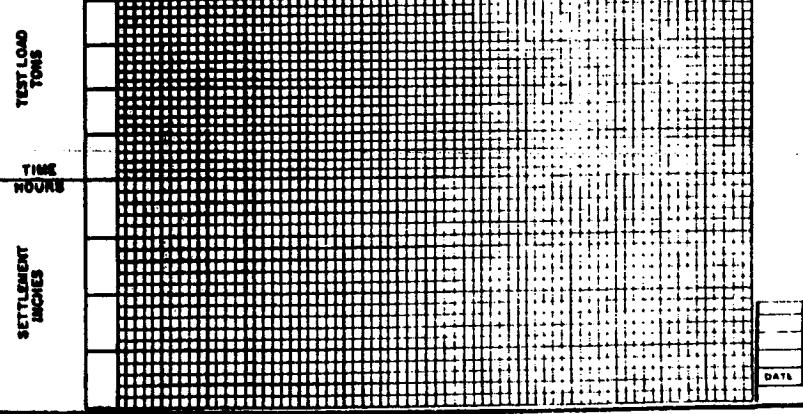
DRIVING RESISTANCE
TONS



SOIL TYPE & COLOR	QU	QT	TEST PILE No.		ELEVATION	STA.
			TYPE OF PILE:	TYPE OF HAMMER:		
GR. CLY. SILT	0.15	HR.	AST 27	SL 12	15	15
W/JTR. 020.5%						
GR. CLY. SOY. SILT	0.10	HR.	AST 23	N. P.	10	
GR. SIC. CL.	0.05	HR.	AST 31	SL 7	5	
GR. CLY. SOY. SILT	1.15	HR.	AST 31	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.80	HR.	AST 32	SL 11	10	
GR. CLY. SI. SA.	1.10	HR.	AST 23	N. P.	15	
GR. CLY. SOY. SILT	0.05	HR.	AST 30	SL 3	5	
GR. SIC. CL.	0.70	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. CLY. SOY. SILT	0.05	HR.	AST 30	SL 3	5	
GR. SIC. CL.	0.70	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.60	HR.	AST 30	SL 6	10	
GR. CL.	0.50	HR.	AST 30	SL 6	10	
GR. CLY. SI. SA.	1.00	HR.	AST 23	N. P.	15	
GR. SIC. CL. W/JTR. SILT	0.60	HR.	AST 30	SL 6	10	
GR. CL.	0.50	HR.	AST 30	SL 6	10	
GR. CLY. SILT	0.80	HR.	AST 27	SL 16	15	

BORING NO. STA: 141.76
DATE MADE: 3-8-71 LOCATION: 102-76

DRIVING RESISTANCE
TONS



NOTES		TOTAL SHEETS	STATE PROJECT	PARISH	SHEET NO.
		52	808-10-03	ACQUISITION	500-24
N.	- Blue	CL.	Clay	Streets	
M.	- Black	GRAV.	General	Driveway	
G.	- Brown	ORG.	Organic	Tree	
G.	- Gray	SA.	Sand	Soil	
PK.	- Green	SH.	Shaly	Plastic	
WH.	- Pink	SI.	Silt	No Cull	
YE.	- Yellow	ALT.	Alternating	Sediment	
CONC.	- Concentric	FI.	Fine	Yield	
RTS.	- Roots	LE.	Medium	Multiple Shear	
ROT.	- Rotten	LAM.	Loamy	Vertical Shear	
VEG.	- Vegetation	LEN.	Loamy	Shear Angle	
WD.	- Weed	SL.	Stony	Stony	
		LY.	Layers	Layers	

MOISTURE CONTENT - Moisture Content of the material in its natural state expressed as a percentage of the dry weight of the material

NO PENETRATION - Location and identification of the sample

NO CULL - Unable to drive split spoon sampler initial 6 inches

NO PREVIOUS DRIVING - Number of blows of 140 lbs. hammer dropped 20 in. required to drive 2 in. O.D. Split Spoon sampler 1 ft. after first having been driven & in. unless amount of penetration shown otherwise

NO PREVIOUS DRIVING - No preliminary 6" driving prior to securing drive data

COMPRESSIVE STRENGTH - Compressive strength from triaxial undrained shear test (tons per sq. in.)

CONSOLIDATED UNDRAINED SHEAR TEST - Compressive strength from consolidated undrained shear test (tons per sq. in.)

ANGLE OF INTERNAL FRICTION - Angle of internal friction

WET WEIGHT OF IN-PLACE MATERIAL - Wet weight per cu. ft.

STRENGTH DETERMINED BY VANE SHEAR TEST - Strength determined by vane shear test

ESTIMATED UNCONFINED COMPRESSIVE STRENGTH BY CORRELATION WITH SIMILAR MATERIAL ON THIS PROJECT - Estimated unconfined compressive strength by correlation with similar material on this project.

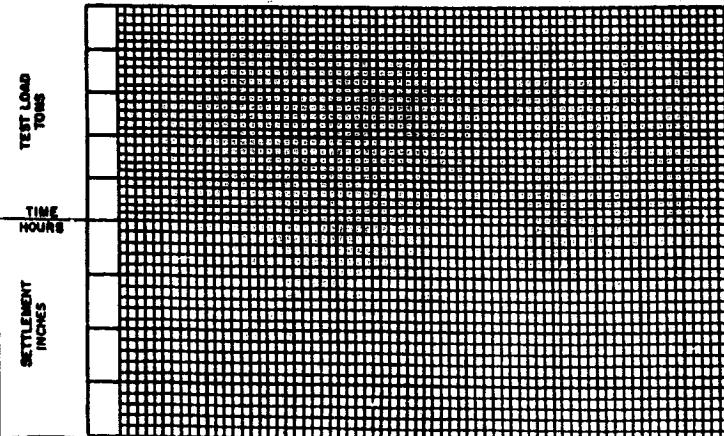
CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES			
SOIL	DESIGNATION	NO OF BLOWS "N"	UNCONFINED COMPRESSIVE STRENGTH TONS PER SQUARE FOOT
SAND & SILT	RELATIVE DENSITY	VERY LOOSE LOOSE MEDIUM DENSE VERY DENSE	LESS THAN 4 4-10 10-30 30-60 OVER 60
CLAY	CONSISTENCY	VERY SOFT SOFT MEDIUM STIFF HARD	LESS THAN 2 2-4 4-6 6-12 12-30 OVER 30

LOCATION (POINT)	STATION	PLAN TIP ELEV.	CUT OFF LINE	PLAN FILE LENGTH	ORDER LENGTH	PILE DATA		
LENGTH	MAX. ELEV.	MIN						

SOIL TYPE & COLOR	TEST PILE No.								
	q	q'	TYPE BREAK	MOISTURE CONTENT	LIQUID LIMIT	PLASTICITY INDEX	SAMPLE NUMBER	ELEVATION	STA.
(W/TR. WB. # 006) (D ZL. 19.4 - 13.7)									
GR. SI. CL. S.	0.18	H.S. 065	-	N. P.	15	15	15	15	STA. 102-465
GR. CL. S.	0.16	H.S. 061	-	N. P.	15	15	15	15	STA. 102-466
GR. SI. CL. W/STR. S.	1.12	H.S. 069	31	37	14	15	15	15	STA. 102-467
H.C.									
GR. SI. PI. S. S.			- N. P.	15	15	15	15	15	STA. 102-468
GR. CL. W/GR. LBN			SI. ST. 15	15	15	15	15	15	STA. 102-469
GR. SI. CL.	1.27	V.L. 066	29	40	19	15	15	15	STA. 102-470
1.48									

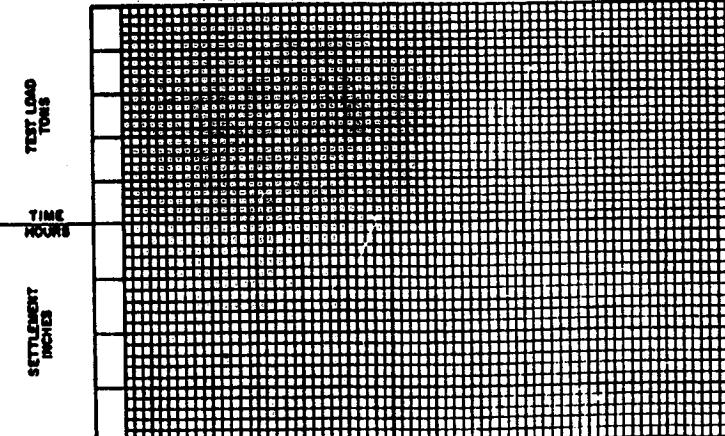
BORING NO. 1
DATE MADE: 4-9-76
LOCATION: 6

DRIVING RESISTANCE
TONS



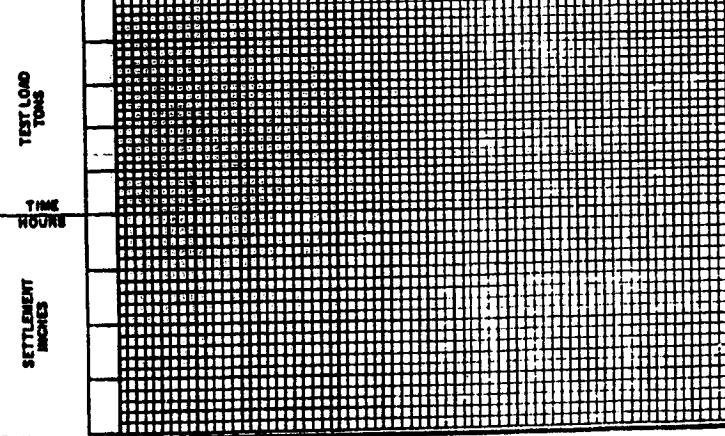
BORING NO. 2
DATE MADE: 5-7-76
LOCATION: 80' AT. 6

DRIVING RESISTANCE
TONS



BORING NO. 3
DATE MADE: 5-7-76
LOCATION: 80' AT. 6

DRIVING RESISTANCE
TONS



NOTES:	STATE PROJECT	MANH.	SHEET NO.
	304-11-06 Assumption	112	23

SL.	Bl.	Clay	STK.	Soil
SL.	Black	C. Crete	M.	Mottled
SL.	Brown	Organic	14.	Iron
SL.	Gray	Sand	N. P.	Non-plastic
SL.	Green	Sh.	PENET.	Penetration
SL.	Pink	SI.	No Cull	
SL.	White	ALT.	Alternating	S.S.
SL.	Yellow	CO.	Coarse	Silt-sand
CONC.	Concrete	FI.	Fine	Yield
RTS.	Roots	MED.	Medium	Multiple shear
ROT.	Rotten	L. O.	Loamy	Vertical shear
VEG.	Vegetation	LEN.	Iron Ore	Shear angle
WD.	Wood	Q. A.	Stump	Slump
				Quick Acting

MOISTURE CONTENT: Moisture Content of the material in its natural state expressed as a percentage of the dry weight of the material

③ NO PENETRATION: Location and identification of the sample

N = 10: Unable to drive split spoon sampler initial 6 inches

NO CULL: Number of blows of 140 lbs. hammer dropped 30 in. required to drive 2 in. O.D. Split Spoon sampler 1 ft. after first having been driven 6 in. unless amount of penetration shown otherwise

No preliminary 6" driving prior to securing drive data

UNCONSOLIDATED UNDRAINED SHEAR TEST: Compressive strength from triaxial undrained shear test (tons per sq. ft.)

CONSOLIDATED UNDRAINED SHEAR TEST: Compressive strength from consolidated undrained shear test (tons per sq. ft.)

ANGLE OF INTERNAL FRICTION: Angle of internal friction

WET WEIGHT OF IN-PLACE MATERIAL: Wet weight of in-place material (Tons per cu. ft.)

STRENGTH DETERMINED BY PENETROMETER: Strength determined by penetrometer

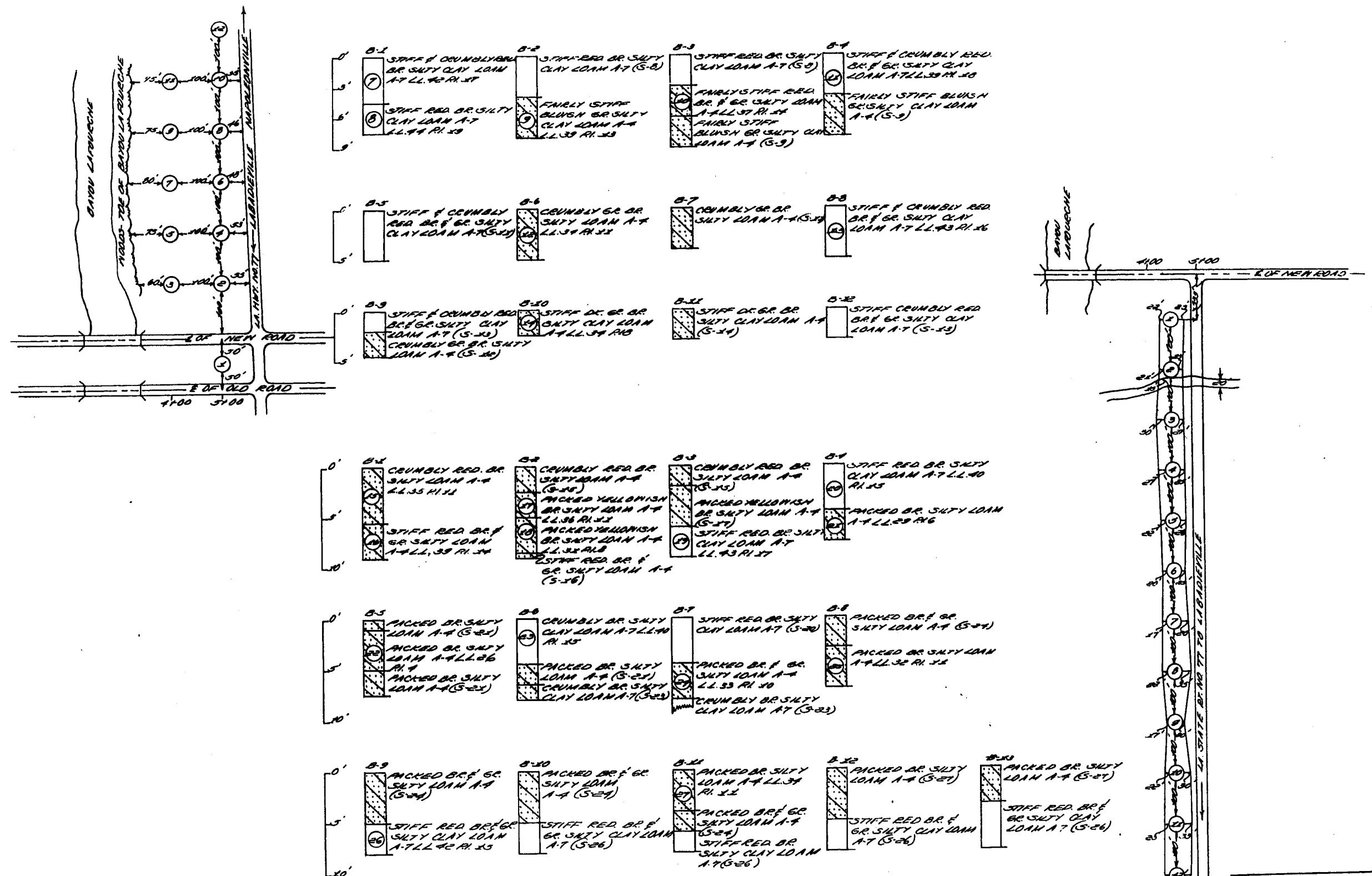
ESTIMATED UNCONSOLIDATED COMPRESSIVE STRENGTH: Estimated unconfined compressive strength by correlation with similar material on site project.

CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES			
SOIL	DESIGNATION	NO OF BLOWS "N"	UNCONSOLIDATED COMPRESSIVE STRENGTH TONS PER SQ.FT.
SAND & SILT	CONSISTENCY VERY LOOSE LOOSE MEDIUM DENSE VERY DENSE	LESS THAN 4 5-10 10-50 30-50 OVER 50	
CLAY	CONSISTENCY VERY SOFT SOFT MEDIUM STIFF VERY STIFF HARD	LESS THAN 2 2-4 4-8 8-15 15-30 OVER 50	LESS THAN 0.25 0.25 - 0.50 0.50 - 1.00 1 - 2 2 - 4 OVER 4

PILE DATA						
LOCATION (IDENT)	STATION	PLAN TIP ELEV.	CUT-OFF ELEV.	PLAN LENGTH	ORDER LENGTH	AS BUILT TIP ELEV.
1	100+78.82	-51.78	10.22	70		
2	100+87.82	-51.64	10.56	70		
3	101+10.82	-51.51	10.49	70		
4	101+56.82	-51.57	10.68	70		
5	101+54.82	-51.24	10.76	70		
6	101+78.82	-51.10	10.89	75		

CORE BORINGS AND TEST PILES		
BAYOU LAFOURCHE BRIDGE AT MUNSON LA. 402		
DATED: JUNE 26, 1976		
STATE OF LOUISIANA DEPARTMENT OF HIGHWAYS		
F.B. NO. SL-3976		
SURVEYED: <input checked="" type="checkbox"/> DETAILED: <input type="checkbox"/> TRACED: <input checked="" type="checkbox"/> CHECKED: <input type="checkbox"/> F.B. NO. <input type="checkbox"/> CHECKED: <input checked="" type="checkbox"/> REVISED: <input type="checkbox"/>		
DATA: DESCRIPTION: BY: MATERIAL SECTION: BRIDGE DESIGN SECTION:		

7



BORROW PIT MAPS

二〇一九年

STATE

SIA DEPARTMENT

DEPARTMENTS

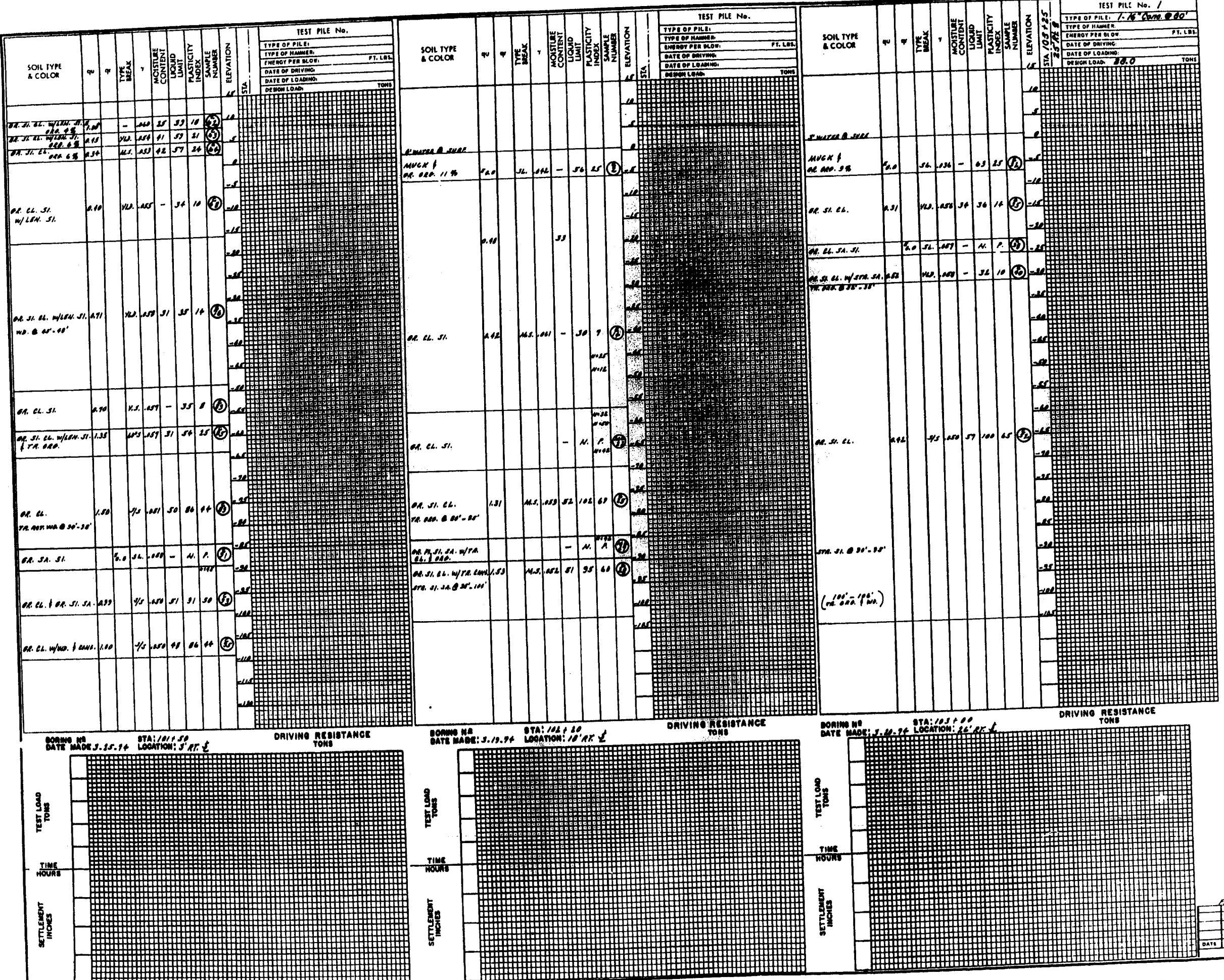
SEARCHED **J.M.**

CHARGE OF H.L. LEHM

410-10-33

**STATE OF LOUISIANA
DEPARTMENT OF HIGHWAYS**

NONED UNF.	DETAILED <i>SPK.</i>	TRACED <i>CON.</i>
SEARCHED <i>JAN.</i>	CHECKED <i>WED.</i>	CHECKED <i>WED.</i>
CHARGE OF M.L. LEHMANN-TESTING & RESEARCH EN		



		A-1 1973-02-01		B-1 1973-02-01		SHEET 12	
		739-02-01 ASSUMPTION					
Blue	C. I.	Clay	Silt	Sands	Mud		
Black	Unst.	Organic	Marl	Shells	Marl		
Brown	Calcareous	Chlorite	Silt	Fines	Iron Oxide		
Gray	SA.	Shale	Cl. P.	Flint	Flint		
Green	St.	Shelly	PEBBLES	Peat	Peat		
Pink	SI.	Silty	LL. CL.	LL. CL.	LL. CL.		
White	AL.	Alternating	S.S.	Slackstones			
Yellow	CO.	Calcareous	Y.L.D.	Yield			
Concretions	FL.	Fine	N.C. S.	M. Triple Shear			
Roots	MUD.	Medium	N.C. S.	Zones of Shear			
Rotten	I. O.	Iron Ore	60% S.	Shear Angle			
Vegetation	LEN.	Lenses	S.I.	Slip			
Wood	O. A.	Quick Acting					
22							
E CONTENT Moisture Content of the material in its natural state - expressed as a percentage of the dry weight of the material							
Location and Identification of the sample							
PERATION Unable to drive split spoon sampler initial 6 inches							
Number of blows of 100 lbs. hammer dropped 30 in. required to drive 2 in. O.D. Split Spoon sampler 1 ft., after first having been driven 6 in., unless amount of penetration shown otherwise							
No preliminary 6" driving prior to recording drive data							
Compressive strength from triaxial unstrained shear test (tons per sq. ft.)							
Compressive strength from consolidated undrained shear test (tons per sq. ft.)							
Angle of internal friction							
Wet weight of in-place material (tons per cu. ft.)							
Strength determined by PENETROMETER							
Estimated unconfined compressive strength by correlation with similar material on this project.							
CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES							
OIL		DESIGNATION		N^o OF BLOWS "N"		"E_{u"} UNCONFINED COMPRESSIVE STRENGTH TONS PER SOFT.	
AND SILT	RELATIVE DENSITY	VERY LOOSE		LESS THAN 4			
		LOOSE		4-10			
CLAY	CONSISTENCY	MEDIUM		10-30			
		DENSE		30-50			
AND SILT	RELATIVE DENSITY	VERY DENSE		OVER 50			
CLAY	CONSISTENCY	VERY SOFT		LESS THAN 2		LESS THAN 0.25	
		SOFT		2-4		0.25-0.50	
CLAY	CONSISTENCY	MEDIUM		4-8		0.50-1.00	
		STIFF		8-15		1-2	
CLAY	CONSISTENCY	VERY STIFF		15-30		2-4	
		HARD		OVER 30		OVER 4	

STATION (BENT)	STATION	PLAN TIP ELEV.	CUT- OFF ELEV.	PLAN PILE LENGTH	ORDER LENGTH	PILE DATA		
						AS BUILT	TIP ELEV.	BL
1	101+57	-32.98	16.02	50'				
2	101+76	-38.98	16.02	55'				
3	101+95	-48.98	16.02	65'				
4	102+14	-53.98	16.02	70'				
5	102+33	-58.98	16.02	75'				
6	102+52	-58.98	16.02	75'				
7	102+71	-58.98	16.02	75'				
8	102+90	-58.98	16.02	75'				
9	103+09	-58.98	16.02	75'				
10	103+28	-48.98	16.02	65'				
11	103+47	-38.98	16.02	55'				
12	103+66	-33.98	16.02	50'				

CORE BORINGS AND TEST PILES		
BAYOU LAFOURCHE BRIDGE (SUPREME)		
LA.1011		
DATED APRIL 17 1974		
STATE OF LOUISIANA DEPARTMENT OF HIGHWAYS		
SURVEYED <i>M. FASTER</i>	DETAILED <i>CHEMEN</i>	TRACED <i>LEWIS</i>
CHECKED <i>M. FASTER</i>		CHECKED <i>LEWIS</i>
MATERIAL SECTION		BRIDGE DESIGN

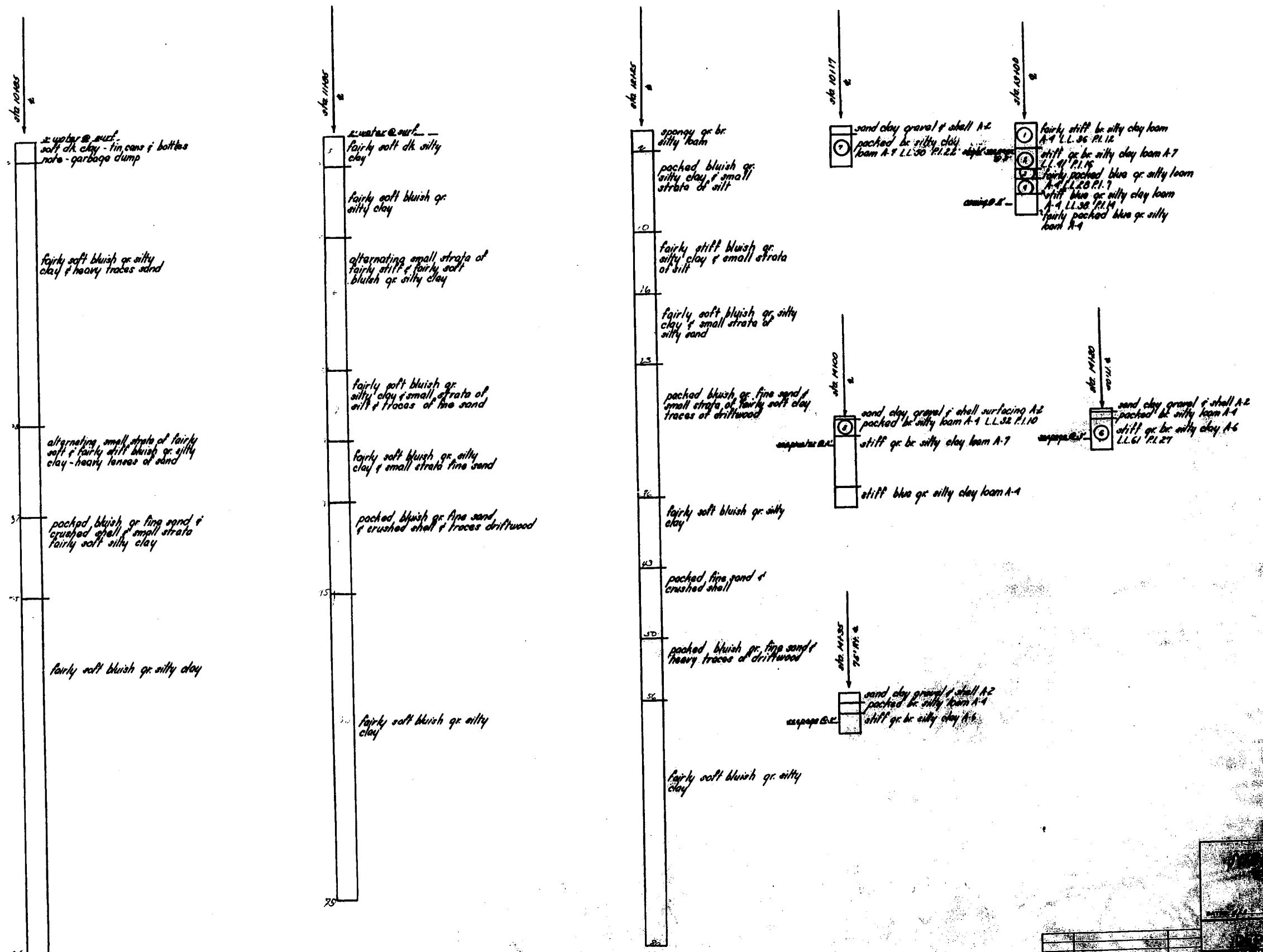
10

BAYOU LAFOURACHE BRIDGE -
LABADIEVILLE
STATE PROJ. 804-35-51
ASSUMPTION PARISH

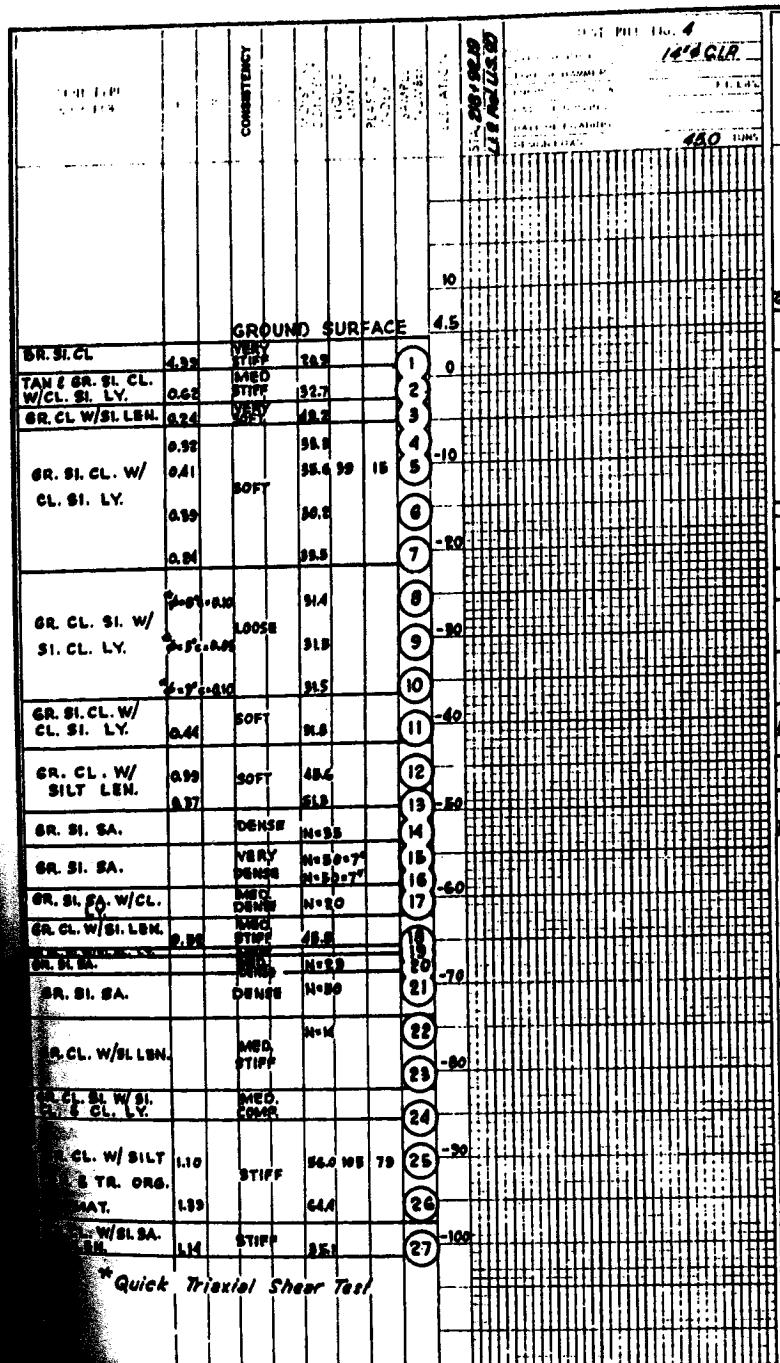
scales:
 legend
 () indicates sample location & identification
 LL indicates liquid limit of material
 PI indicates plastic index of material
 A-1 indicates subgrade soil group
 NL indicates non-plastic material
 water: indicates level of free water
 indicates boring location & identification

Field book no 30-822

note:
 ground elev. 0
 unless otherwise
 shown.



DATE	DESCRIPTION	REV.



W C-121 STA: 219+00
MADE 7/18/67, 9/69 LOCATION: 50'

ME C-121 STA: 219+00
MADE 7/26/67, 8/69 LOCATION: 30' LT.

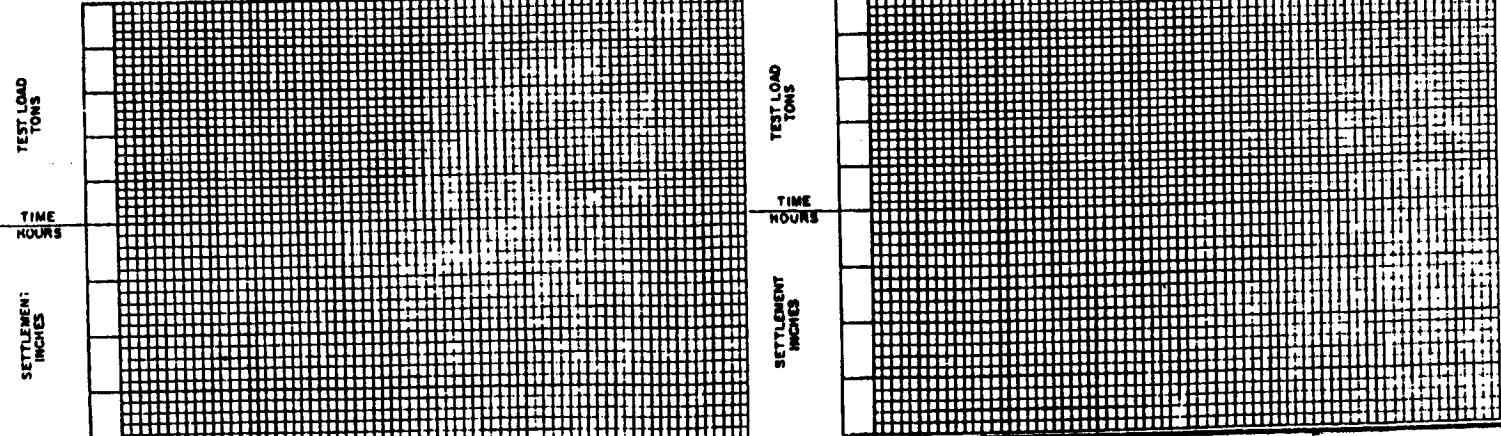
**DRIVING RESISTANCE
TONS**

BORING NO C-121 Cont. STA: DATE MADE: LOCATI

**DRIVING RESISTANCE
TONS**

BORING NO. STA.
DATE MADE: LOCATION:

**DRIVING RESISTANCE
TONS**



NOTES		PAP	WIND LOAD	SOIL TEST	LAPOURCHE	210
		04-04002	424-08-08		LAPOURCHE	210
BL.	Blue	C	100	100	FRAG.	Fragments
BR.	Brown	CB	100	100	W/	White
GR.	Green	CG	100	100	POC.	Plastic
GY.	Grey	CGY	100	100	MAT.	Matter
GR.	Grey	G	100	100	COMP.	Compost
WH.	White	W	100	100		
CONC.	Concrete	CONC	100	100		
STEEL	Steel	STEEL	100	100		
BIT.	Bitumen	BITUMEN	100	100		
ROI.	Rubber	ROI	100	100		
VEG.	Vegetation	VEGETATION	100	100		
UD.	Urban	UD	100	100		
MONITOR CONTENT		Monitor content is determined by weight of each component in the sample.				
(3)		Location of monitor content in the sample.				
NO PUNCTURE		Double or triple thicknesses of material are used.				
N-16		Number of layers of 1/16" thick material required to penetrate 2 mm. of D. I. water at a pressure of 100 kg/cm ² . The number of layers is determined by the following formula:				
NO CULL		No preference of no cull material is given.				
4%		Compressive strength of cull material is determined by the following formula:				
4%		Compressive strength is measured in kilobars. Results are expressed per cent. (P.)				
8		Angle of internal friction.				
		Wet weight of sample measured in kg per sq. m.				
		Strength and permeability are determined.				
1.4'		Estimated wet weight, compressive strength by test, angle of internal friction measured in dry sample.				
CORRELATION OF PENETRATION RESISTANCE AND SOIL PROPERTIES						
SOIL	DESIGNATION	N° OF BLOWS "N"	"B" UNCONFINED COMPRESSIVE STRENGTH TONS PER SQ. FT.			
BAND S. BILT	RELATIVE DENSITY	VERY LOOSE LOOSE MEDIUM DENSE VERY DENSE	LESS THAN 4 4-10 10-30 30-50 OVER 50			
CLAY	COMPATENCY	VERY SOFT SOFT MEDIUM STIFF VERY STIFF HARD	LESS THAN 2 2-5 5-75 15-30 OVER 30	LESS THAN 0.25 0.25-0.50 0.50-1.00 1-2 2-4 OVER 4		

**NOTE: The Test Pile Cut-Off Elevation Is 6.0' (Approx) Above
The Existing Ground.**

ORE BORINGS AND TEST PILES

**RELOCATION OF U.S. 90
(RACELAND - GIBSON)**

**EUSTIS ENGINEERING COMPANY
SOIL AND FOUNDATION CONSULTANTS**

STATE OF LOUISIANA
DEPARTMENT OF HIGHWAYS

DEPARTMENT OF HIGHWAYS

CHEERLEADER **CHEERLEADER**

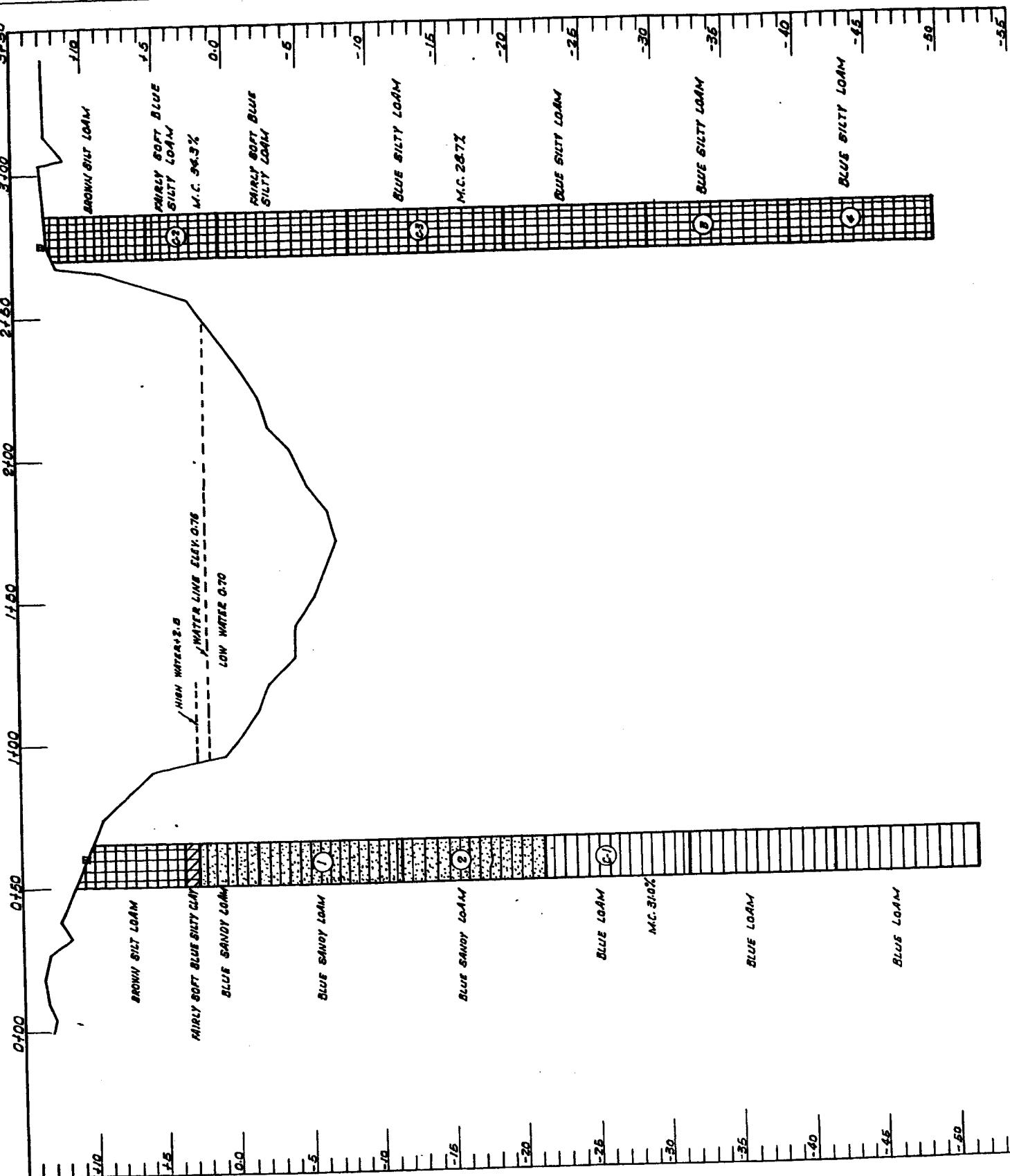
TESTING SECTION — BRIDGE DESIGN SECTION

FIGURE 7

FIGURE 7

**SOIL SURVEY
STATE PROJ. 931-00-23
BAYOU LAFOURCHE PONTOON BRIDGE
LAFOURCHE PARISH**

三



FINAL TRACINGS



Note

- ... CENTER OF DRILLING
- M.G. 23.6% ... Motions Content of Sample
- (1) ... CORE SAMPLE NUMBER & LOCATION
- (2) ... SAMPLE # & Location (disturbed cores)

